



# Proposed RACF AT 64 WARNERS BAY ROAD, WARNERS BAY

## Stormwater Drainage Report

### Civil Engineering Consulting Services



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## Appendices

- A Water Quality Treatment Train

# **1. Introduction**

## **1.1 Background**

MPC Consulting Engineers has been engaged to formulate a stormwater drainage report to support a Development Application for a proposed residential aged care facility at 64 Warners Bay Road, Warners Bay NSW. The drainage report addresses both water quality and water quantity requirements for the proposed development. The drainage report has been undertaken as the proposed development site is larger than the limit of Lake Macquarie City Councils standard on-site detention calculations spreadsheet.

## **1.2 Study Area**

The proposed development site is approximately 14423 m<sup>2</sup> in area and comprises Lot 1, 2, 3 and 4 DP515512 & Lot 11 DP 656806. The site is adjacent to South Creek. The site in its current state is subject to flooding in the 1 in 100 ARI and the PMF Storm events.

## **1.3 Aims**

### **1.3.1 Water Quality**

To provide adequate water quality control facilities for the site to minimise the effects of the development on the receiving waterway, and to ensure that the existing level of water quality present on the site is not detrimentally affected by the proposed development

### **1.3.2 Water Quantity**

To provide adequate detention facilities such that the Post Development flow is limited to 100% of the Pre-Development flow for Average Recurrence Intervals (ARI) from 5 to 100 years.

## **1.4 Flooding**

MPC understand that the site is subject to the 1 in 100 year and PMF flood events. The proposed building and carpark levels have been adopted to be above the 1 in 100 year flood event, and the building levels have been designed to achieve a 500mm freeboard to the PMF flood event. The floor levels are based on the recommendation in the Northrop report NL151532.

In addition to keeping floor levels above these flood events, the building and a portion of the carpark structure will be designed to incorporate floor inundation below the structure in these flood event to minimise any effects of flooding on neighbouring properties as a result of this development.

## 2. Proposed Water Quantity and Water Quality Measures

### 2.1 Proposed Water Quality Measures

To simulate the current and proposed stormwater quality discharging from the site area, MPC has used the Model for Urban Stormwater Improvement Conceptualisation Version 3.01 (MUSIC). MUSIC has been developed by the Cooperative Research for Catchment Hydrology (CRC) and can simulate a range of water quality features working as part of a treatment train at 6 minute intervals. MUSIC is a water quality modelling software that uses historical data to estimate the effectiveness of a stormwater quality treatment network for a number of different land uses.

MUSIC has a number of in-built modules that simulate both the generation of pollution from different land uses and the treatment and removal of pollution.

For this study, 6 minute rainfall data from the Bureau of Meteorology was used in the model.

The existing conditions for stormwater quality were modelled as a forest sub-catchment, using default pollutant export parameters.

The treatment train for the catchment consists of a rainwater tank for each roof area and a Gross Pollutant Trap located on the outflow of each detention system.

The results of the MUSIC model are given in the following table.

Table Estimated Pre-Development Pollutant Export Rates

Pollutant (annual load kg/yr)	Source	Residual Load	% Reduction
Suspended Solids	736	108	85.3
Total Phosphorous	1.49	0.090	94
Total Nitrogen	10.5	1.62	84.7
Gross Pollutants	122	0	100

The treatment train of the MUSIC model for the catchment is located in Appendix A. The MUSIC file has been issued by email.

### 2.2 Proposed Water Quantity Measures

It is proposed to use three underground detention tanks to cater for the discharge from the site. The proposed location of the detention tanks is shown on MPC's stormwater drawings.

The system supplied will ensure that the total discharge from the development will not exceed that prior to development.

### 3. Hydrological Analysis for Discharge Calculations

#### On-Site Harvesting

In regard to stormwater run-off from the site, on review of Lake Macquarie City Councils DCP. Generally, stormwater run-off will be directed to the stormwater drainage easement after harvesting systems and subsequent detention systems to limit the flows to that of the undeveloped site.

Onsite stormwater harvesting has been designed based on the requirements of the development. The harvesting system is proposed to only be used for irrigation purposes only.

#### Site Parameters:

Site Area =14423 m<sup>2</sup>

Impervious Roof Area (Harvested / Detained) = 5162 m<sup>2</sup>

Impervious Driveway areas (Detained) = 1310 m<sup>2</sup>

Pervious Driveway Area (Permeable Paving) = 315m<sup>2</sup>

It is proposed to incorporate rainwater re-use systems as site harvesting measures which will be used for irrigation purposes around the site. Over flows from the harvesting tank will be directed to the On-site detention system.

#### Site Discharge Index

Site Area (S) – 14423 m<sup>2</sup>

Roof Area (R) – 5162m<sup>2</sup>

Impervious Paved Area (P) – 1310m<sup>2</sup>

Pervious Paved Area (P) – 315m<sup>2</sup>

Total Impermeable Area (I = R+P) – 6787m<sup>2</sup>

DC = I-M = 6787m<sup>2</sup>-5162m<sup>2</sup>-315m<sup>2</sup> = 1310m<sup>2</sup>

SDI = DC/S = 1310m<sup>2</sup> / 14423m<sup>2</sup> = 0.09 < 0.1 Therefore SDI okay

#### On-Site Harvesting

On-site harvesting has been calculated using the mitigation depth procedure outlined in LMCC DCP No.1. A mitigation storage depth of 7mm has been adopted for the Cardiff area using a soil texture of Clay based on the parameters of the geotechnical report indicating that there is clay below the topsoil at the site.

Therefore:

$$MS = MIC \times MD / 1000$$

MIC = Managed Impermeable Catchment (m<sup>2</sup>) – 5162m<sup>2</sup>

MD = Mitigation Depth (mm) – 7mm

$$\underline{MS = Mitigation Storage (KL) = 36.1\text{kl}}$$

A rainwater harvesting tank with a volume of 37000 litres will be provided for the proposed development which will be used for landscape irrigation only.

### On-Site Detention

The catchment was analysed for tank sizing purposes using computer program RARE Version 1.53 which calculates the pre and post development discharge based on the rational method using AR&R kinematic wave equations for times of concentration. Variations to the co-efficient of runoff for intensity 10yr 1hr < 70mm/hr are automatically calculated using triangular hydrographs

Stormwater detention has been provided based on the following calculations.

Outflow from the detention areas will be by way of orifice plate chokes for 1 in 5, 1 in 20 year and 1 in 100 year storm events as required. Surcharge grates allowing for overflow in a fully blocked design case will also be provided with all overflow directed off the site.

### 3.1 Modelling Parameters

The input into RARE is as follows:

**Site Area = 14423m<sup>2</sup>**

**% through OSD = 59%**

**% Impervious pre-developed = 0%**

**% Impervious post-developed = 45%**

**Flow Length = 78m, Slope = 3.8%, Roughness = 0.2**

ARI	Pre- Developed				Post-Developed			Excess Q Post-Pre (l/s)	Vol (m <sup>3</sup> )
	Tc	C	I (mm/hr)	Q (l/s)	Tc	C	I (mm/hr)		
5	16	0.40	85	133	12	0.61	97	37	72.1
10	15	0.42	101	167	11	0.64	116	46	84
20	14	0.44	123	215	10	0.67	59	59	99

50	<b>13</b>	<b>0.48</b>	<b>155</b>	<b>300</b>	<b>10</b>	<b>0.73</b>	<b>92</b>	<b>92</b>	<b>124.4</b>
100	<b>12</b>	<b>0.50</b>	<b>185</b>	<b>370</b>	<b>9</b>	<b>0.77</b>	<b>110</b>	<b>110</b>	<b>142.8</b>

For a 1 in 100 year storm event, it is intended to provide 145 m<sup>3</sup> of storage in an underground detention tanks with an outflow control chamber utilising multiple orifice plate chokes as described above. Orifice plates will be designed as part of the detailed design.

It is proposed to provide one rainwater retention tank adjacent to the main stormwater detention tank to allow for irrigation purposes only for the site. It is understood that no re-use water for toilets or laundry is proposed for the development.

Retention tank and detention tank locations and sizes can be found on MPC's stormwater plans.

## 4. Results

The models for the catchments were run for various design storm durations. The peak discharges for the various ARI's were determined along with the required detention volumes. Critical time of concentration is also shown in the table below. The peak flow results for the developable site are shown in Table 1.

**Table 1: Peak Discharge for Developable Site**

<i>ARI (Years)</i>	<i>Peak Flow (undeveloped) (m³/s)</i>	<i>Peak Flow (Developed) (m³/s)</i>	<i>Time of Concentration <math>T_c</math>(mins)</i>
20	0.215	0.573	14
100	0.370	0.881	12

Table 2 shows the detention volume required for the various ARI's

**Table 2: Detention Requirement**

<i>ARI (Years)</i>	<i>Detention Required (m³)</i>
20	100
100	145

It is proposed to construct 2 separate underground detention tanks throughout the development to have a total volume of 145m<sup>3</sup>.

## 5. Summary and Conclusions

This stormwater drainage report for the proposed Residential Aged Care Facility at 64 Warners Bay, Road, Warners Bay NSW. It is proposed to use a Gross pollutant trap, first flush devices and underground detention tanks for the management of stormwater quality and quantity from the developed site.

## **Figures**

**64 WARNERS BAY ROAD, WARNERS BAY NSW**

**DEVELOPMENT APPLICATION**

**ROADWORKS & DRAINAGE PLANS FOR**

**BUPA WARNERS BAY**

**LOT 1, 2, 3 & 4 DP 515512 & LOT 11 DP 656806**

## SCHEDULE OF DRAWINGS

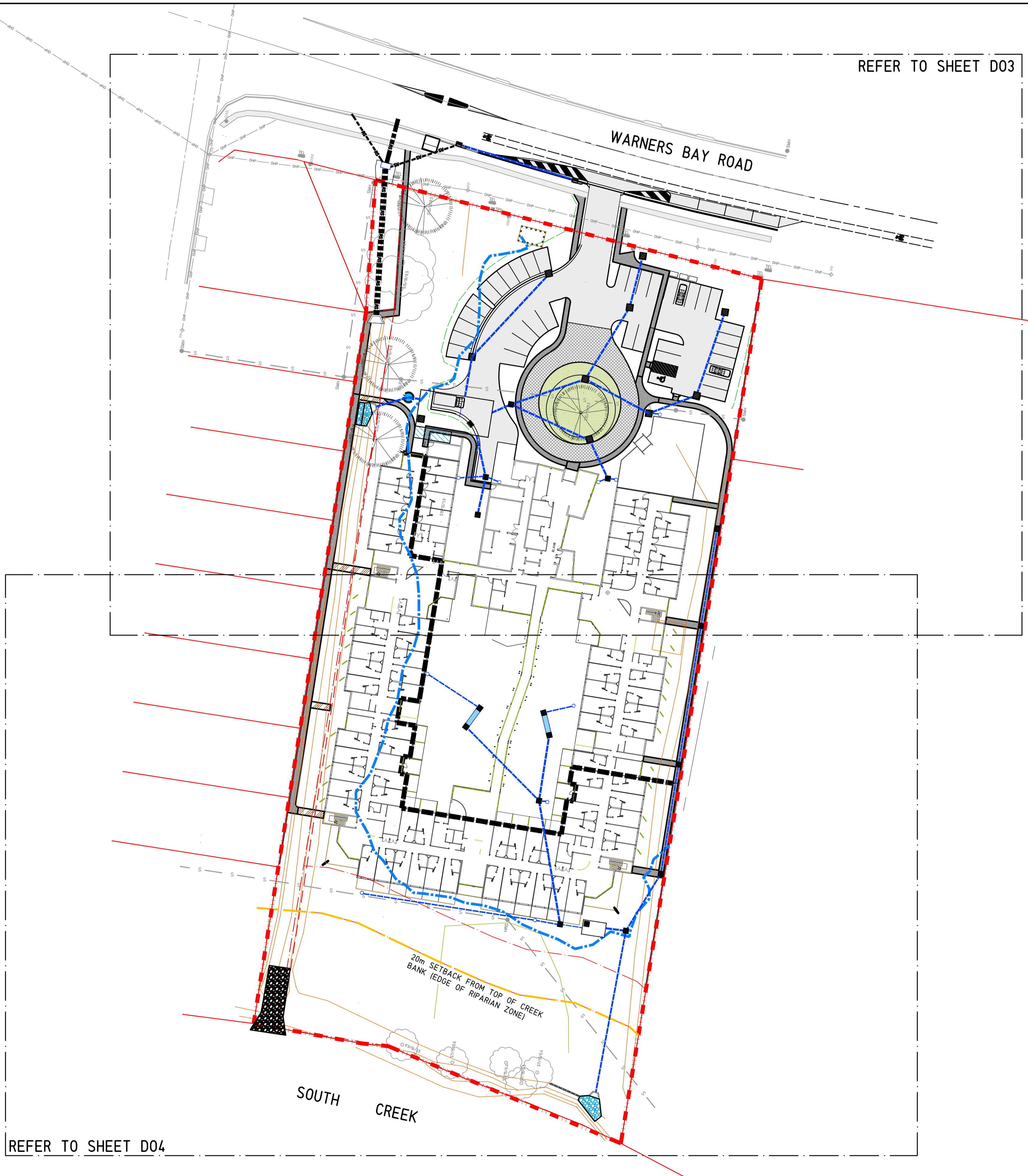
- C01 COVER SHEET & LOCALITY
  - C02 SITE LAYOUT PLAN
  - C03 GENERAL ARRANGEMENT PLAN SHEET 1 OF 2
  - C04 GENERAL ARRANGEMENT PLAN SHEET 2 OF 2
  - C05 TYPICAL SECTIONS & DETAILS
  - C06 ROAD LONGITUDINAL SECTIONS & KERB RETURN PROFILES
  - C07 SITE EARTHWORKS PLAN
  - C08 STORMWATER DRAINAGE PLAN
  - C09 STORMWATER LONGITUDINAL SECTIONS - SHEET 1 OF 2
  - C10 STORMWATER LONGITUDINAL SECTIONS - SHEET 2 OF 2
  - C11 EROSION & SEDIMENTATION CONTROL PLAN



# LOCALITY

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00	CONCEPT PLANS ISSUED FOR CLIENT REVIEW	19.10.2015																				
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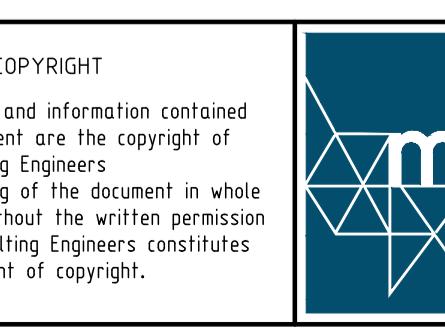
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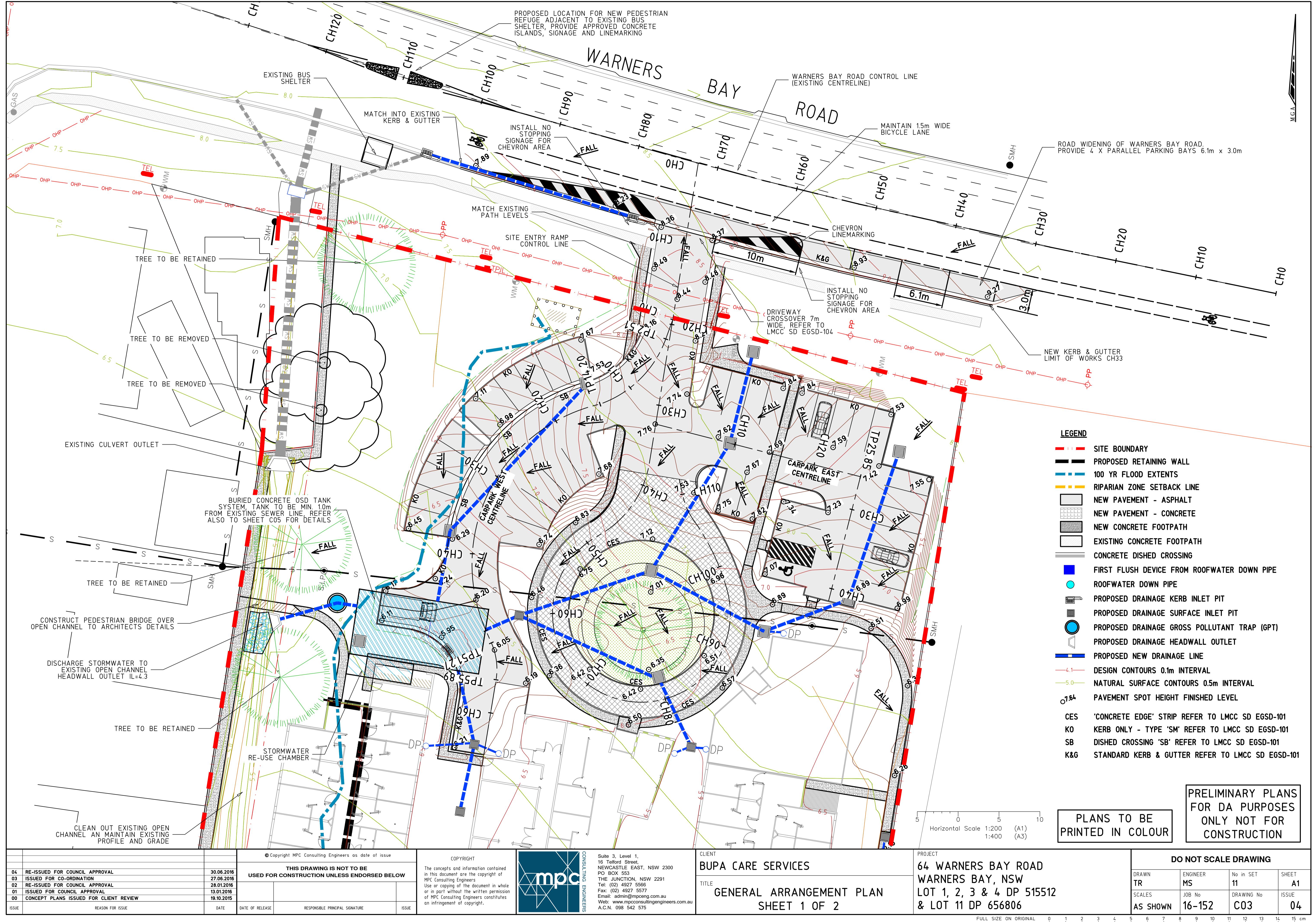
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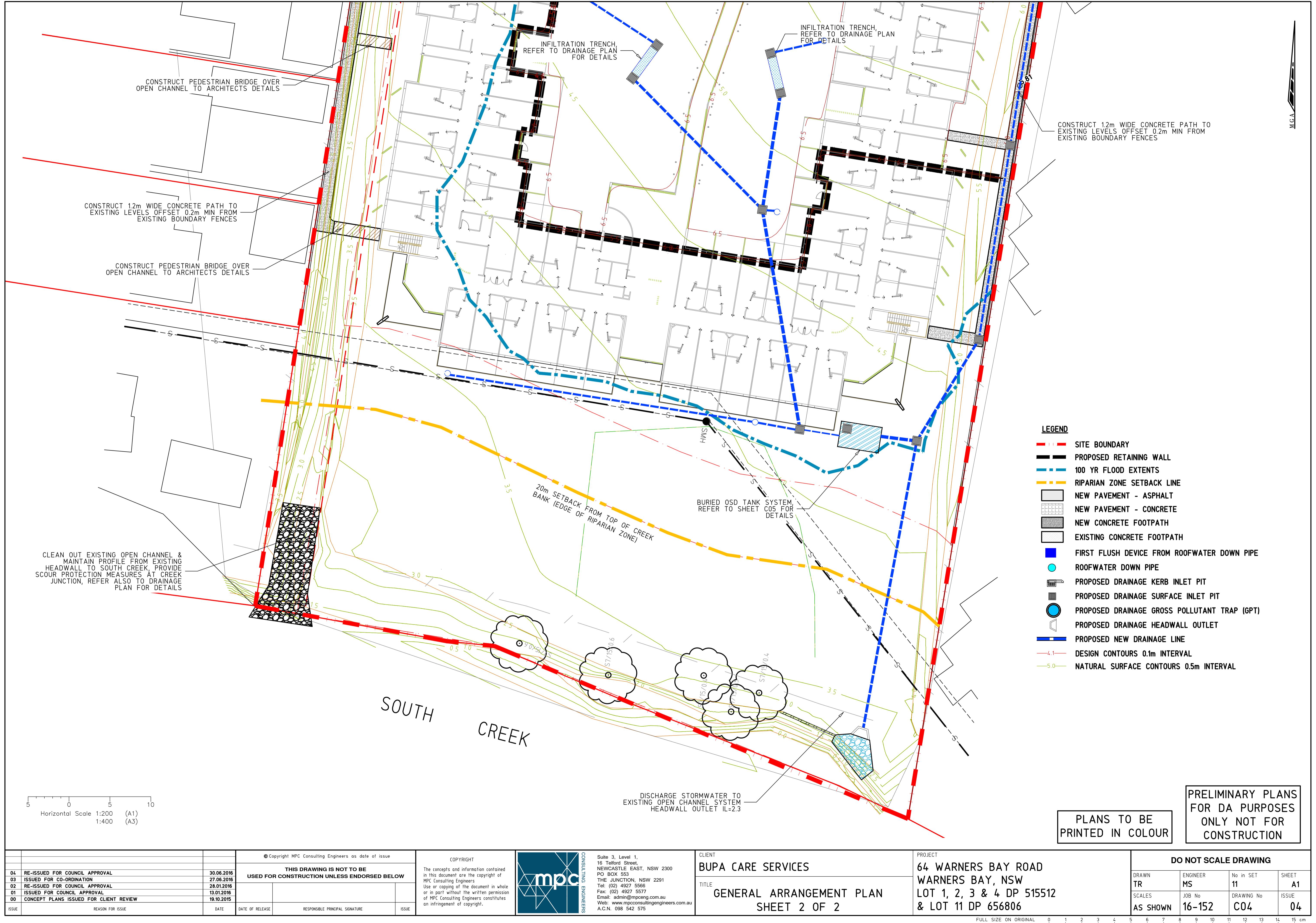
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**& LOT 11 DP 656806**

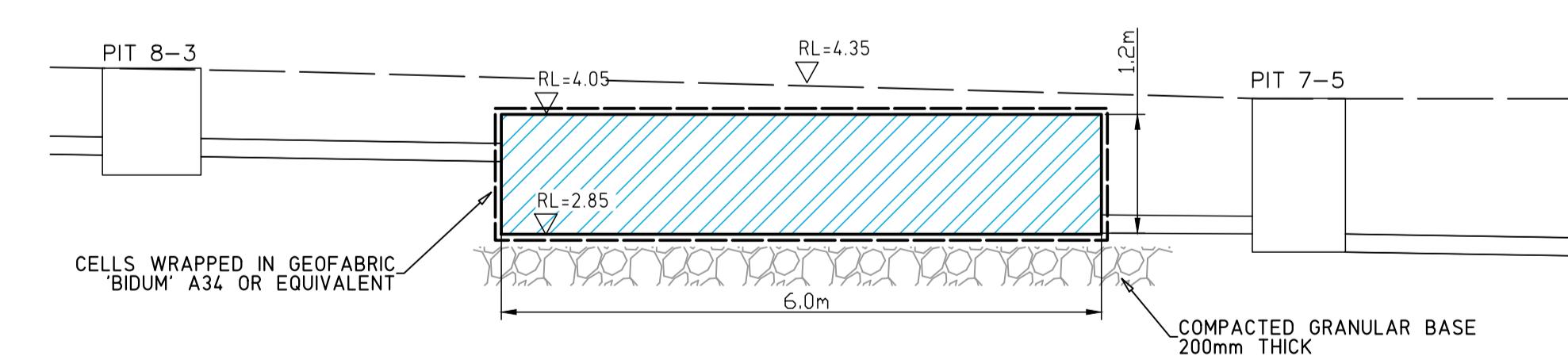
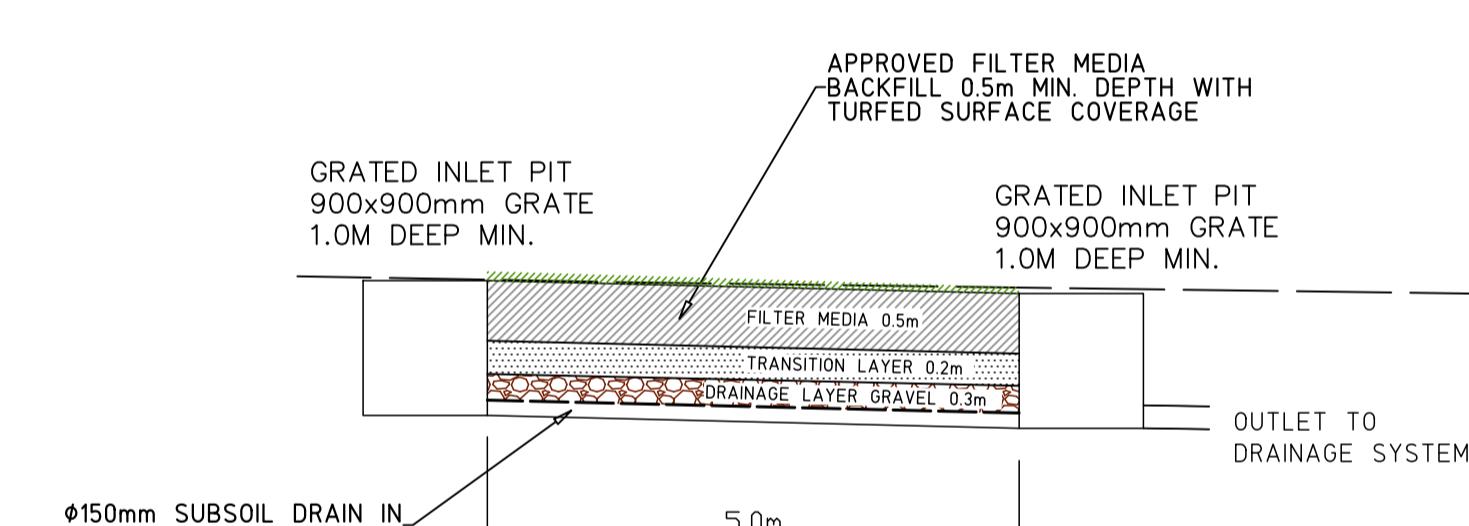
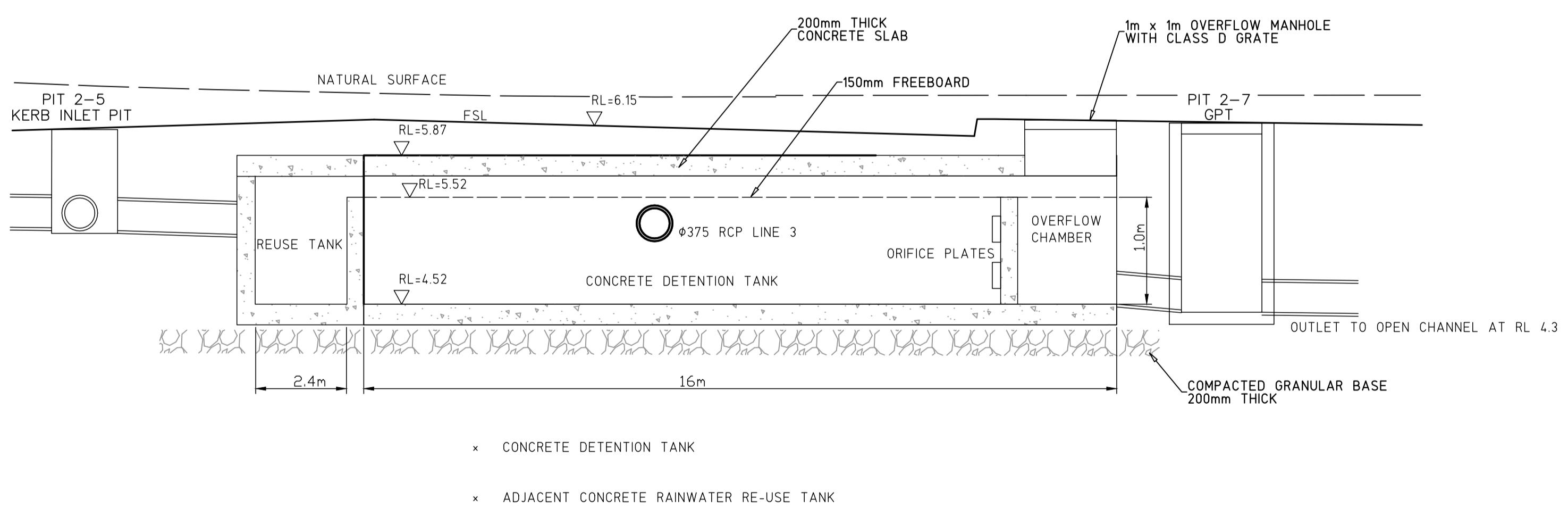
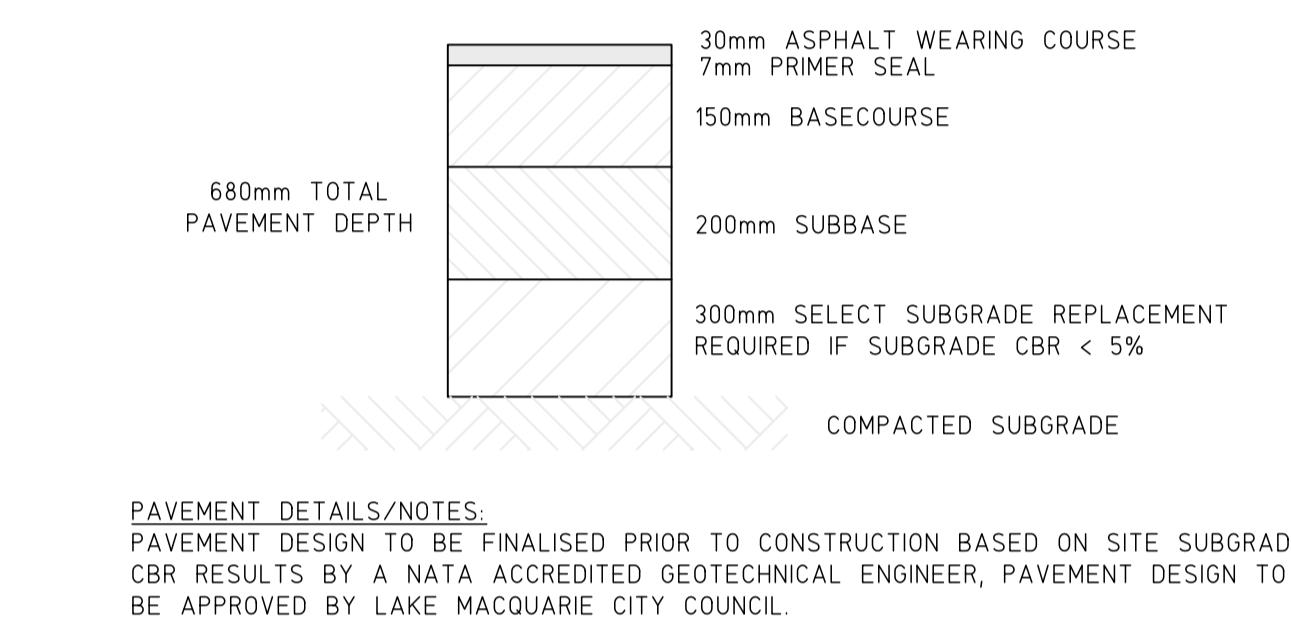
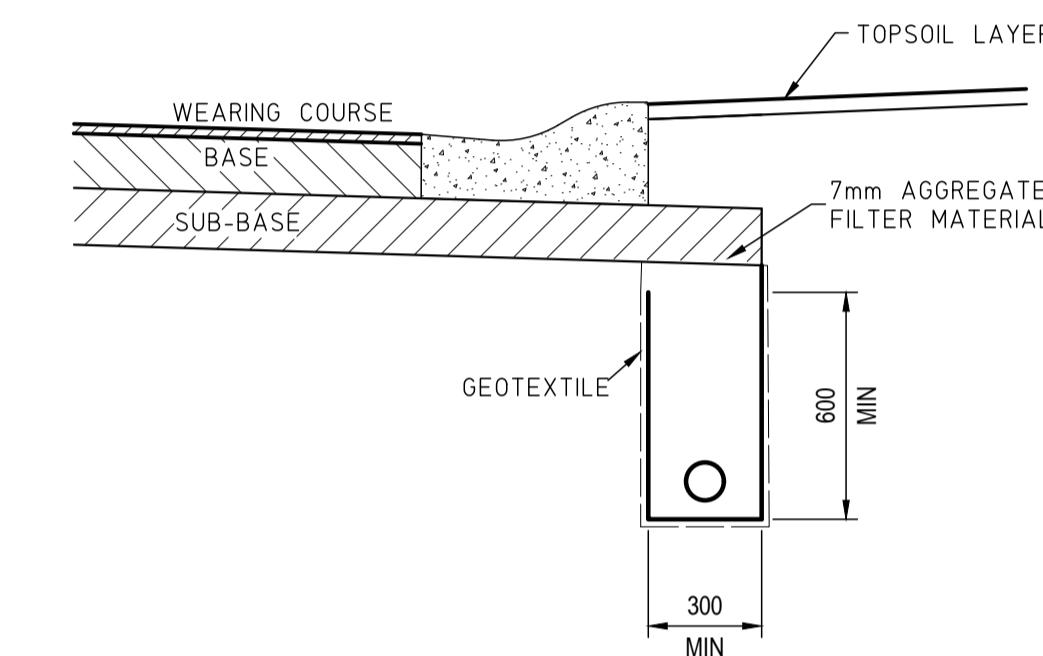
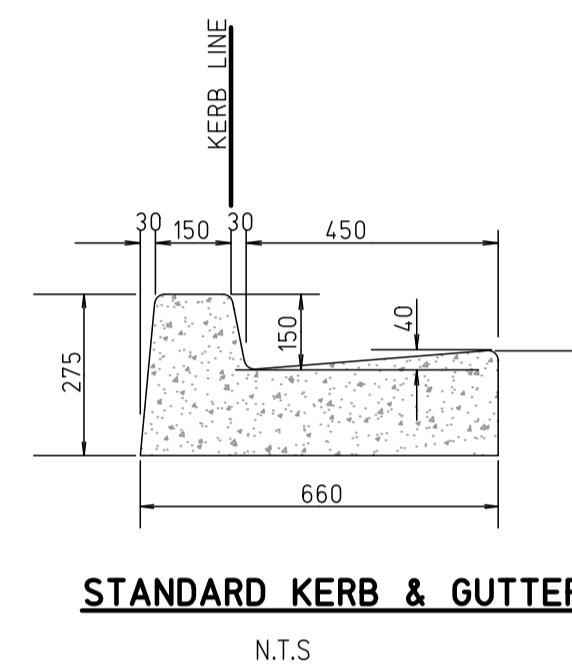
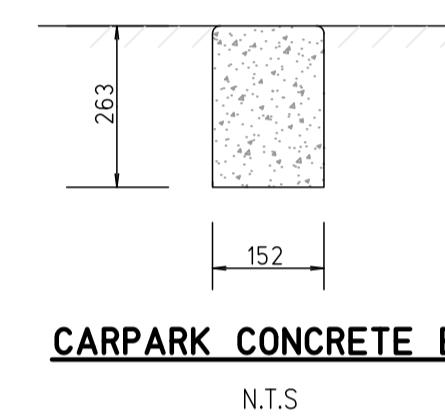
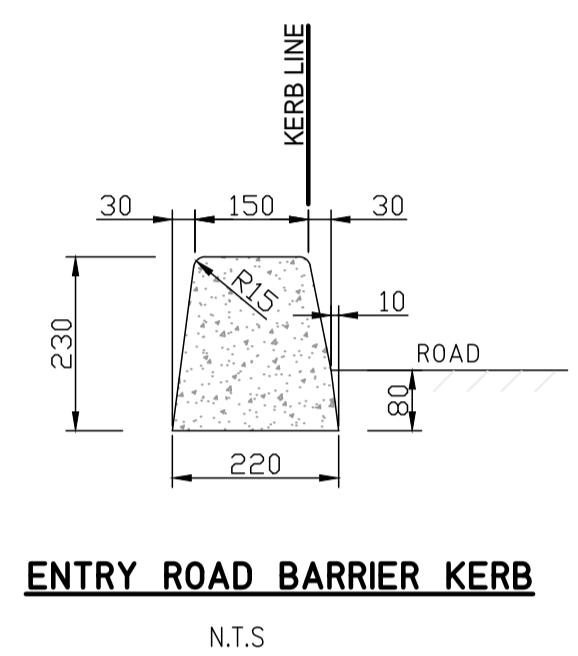
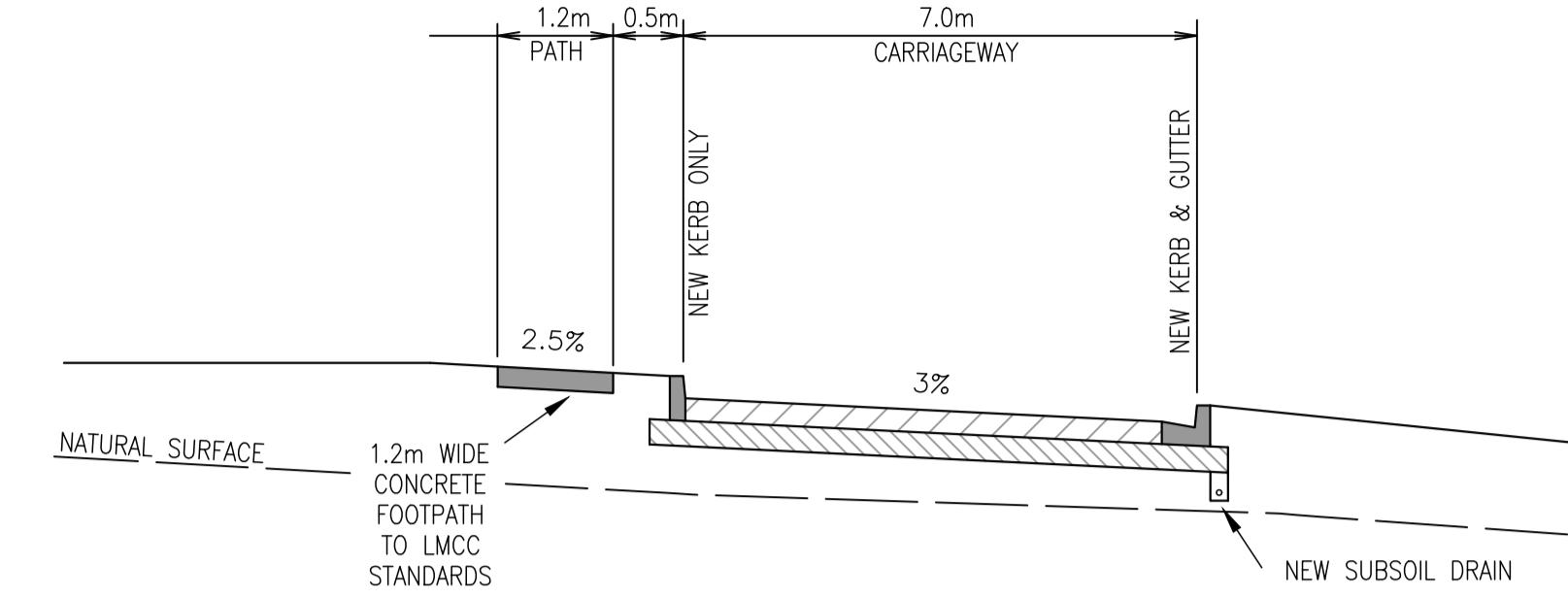
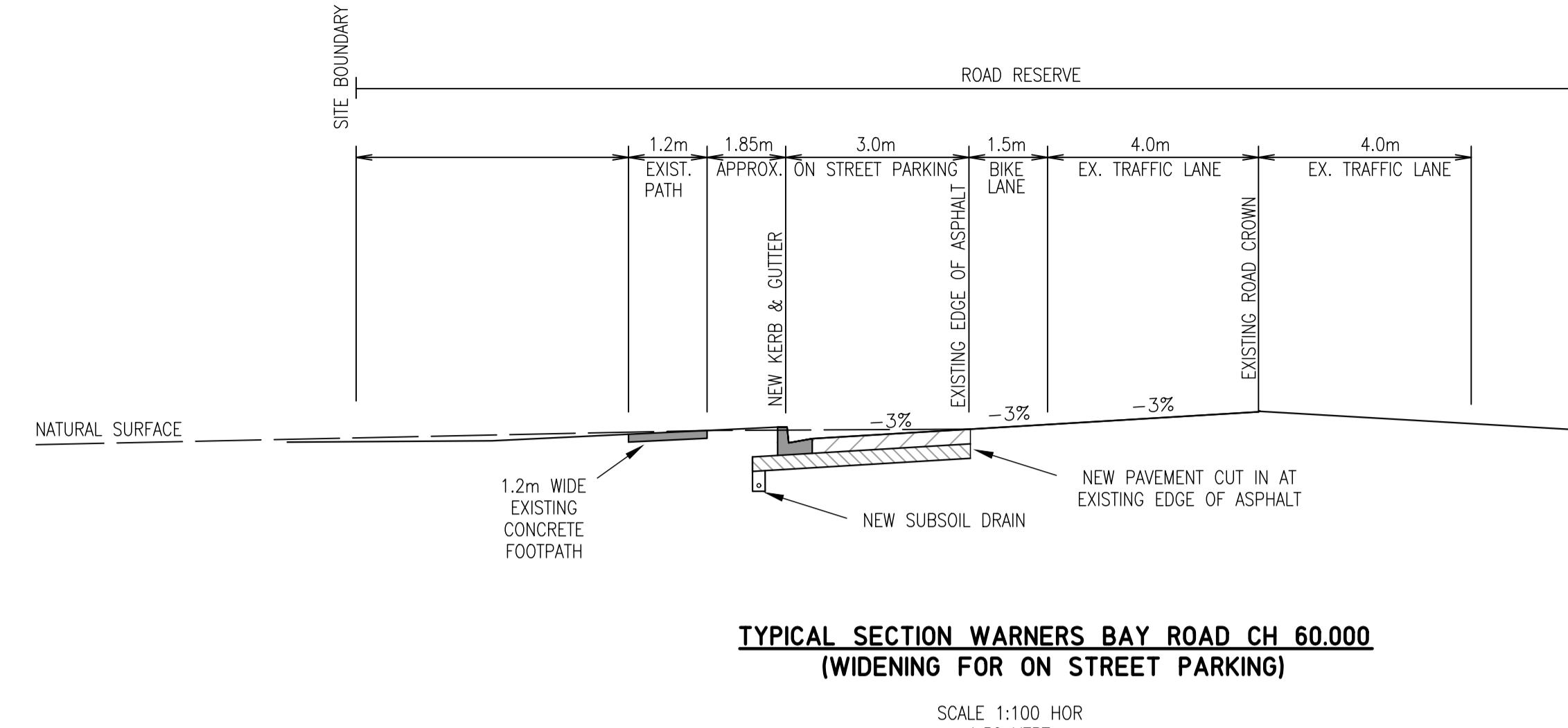
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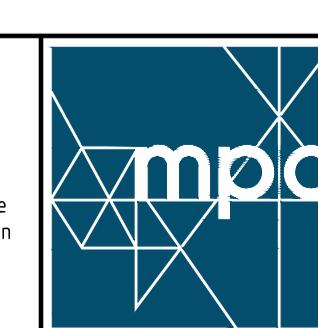
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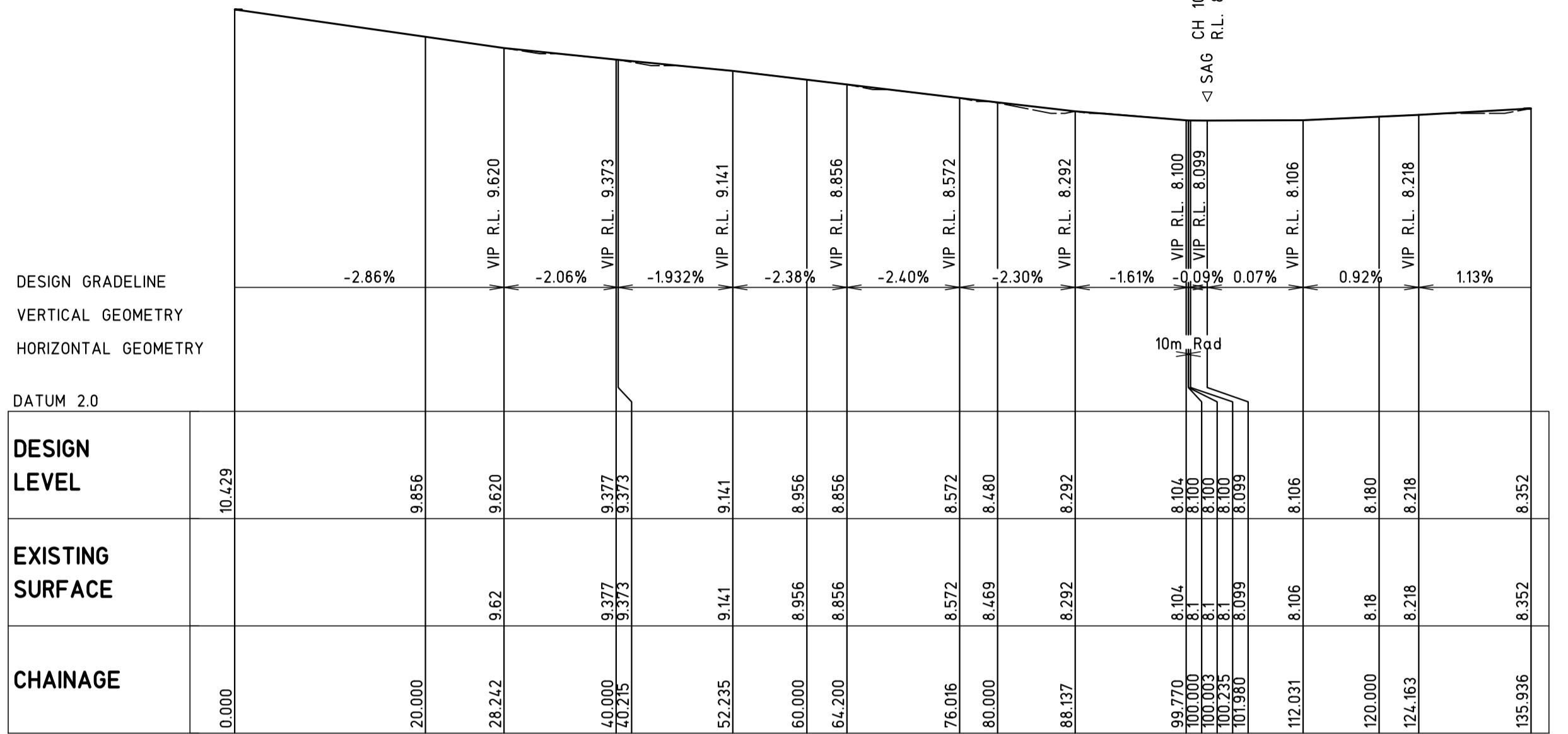
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**TITLE**  
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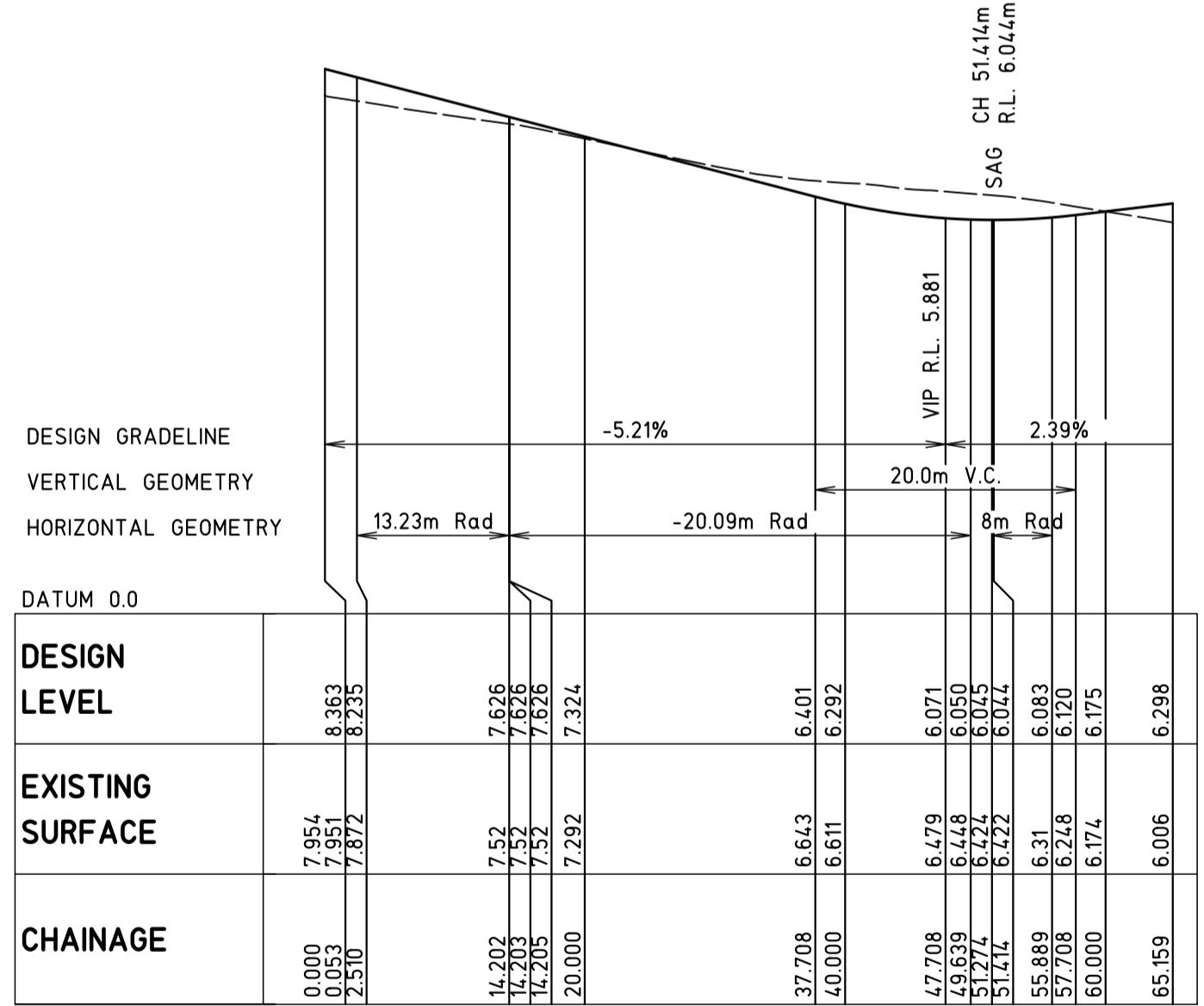
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LONGITUDINAL SECTION - WARNERS BAY ROAD CONTROL LINE (EXISTING SURFACE)

Horizontal scale 1:500  
Vertical scale 1:100



LONGITUDINAL SECTION - CARPARK CENTRELINE (WEST)

Horizontal scale  
Vertical scale 1:

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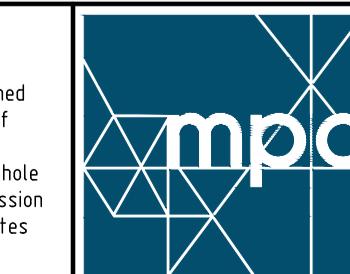
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Table 1. Summary of the main characteristics of the four groups of patients.

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The logo consists of two horizontal black bars. The top bar contains the word 'CLIENT' in a bold, sans-serif font. The bottom bar contains the words 'BUPA CARE SERVICES' in a large, bold, sans-serif font.

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TITLE

## ROAD LONGITUDINAL SECTIONS & KERB RETURN PROFILES

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**WARNERS BAY, NSW**  
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**& LOT 11 DP 656806**

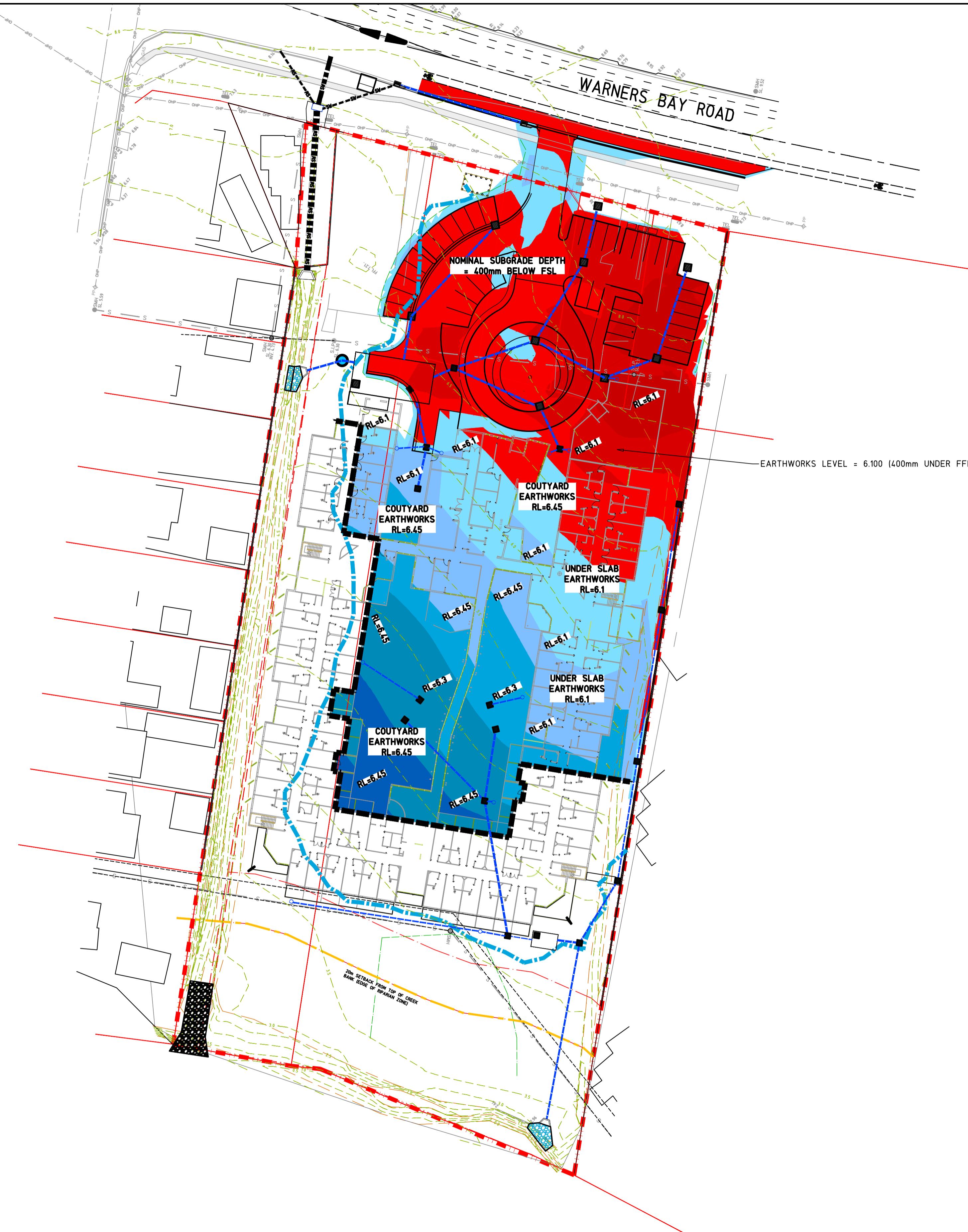
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MGA



**PRELIMINARY EARTHWORKS QUANTITIES FROM NOMINAL STRIPPED SURFACE TO DESIGN EARTHWORKS PADS & NOMINAL PAVEMENT SUBGRADE LEVEL**

TOPSOIL STRIPPING = 1,550m<sup>3</sup> (NOMINAL 200mm STRIP DEPTH CUT TO STOCKPILE)  
 TOTAL CUT FROM STRIPPED SURFACE = 1,690m<sup>3</sup>  
 TOTAL FILL FROM STRIPPED SURFACE = 3,985m<sup>3</sup>  
 BALANCE = 2,295m<sup>3</sup> SHORTFALL

NOTE: NOMINAL PAVEMENT DEPTH 400mm SUBJECT TO FINAL PAVEMENT DESIGN & COUNCIL APPROVAL.

**LEGEND**  
**EARTHWORKS CUT / FILL DEPTH RANGES**

CUT 1.5 - 1.0m DEPTH
CUT 0.5 - 1.0m DEPTH
CUT 0.0 - 0.5m DEPTH
FILL 0.0 - 0.5m DEPTH
FILL 0.5 - 1.0m DEPTH
FILL 1.0 - 1.5m DEPTH
FILL 1.5 - 2.0m DEPTH
FILL 2.0 - 2.5m DEPTH

<b>LEGEND</b>	
SITE BOUNDARY	
PROPOSED RETAINING WALL	
100 YR FLOOD LEVEL EXTENTS	
NATURAL SURFACE CONTOURS 0.5m INTERVAL	

Horizontal Scale 1:500 (A1)  
1:1000 (A3)

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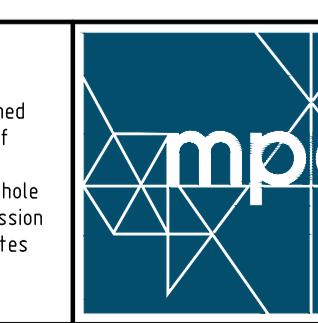
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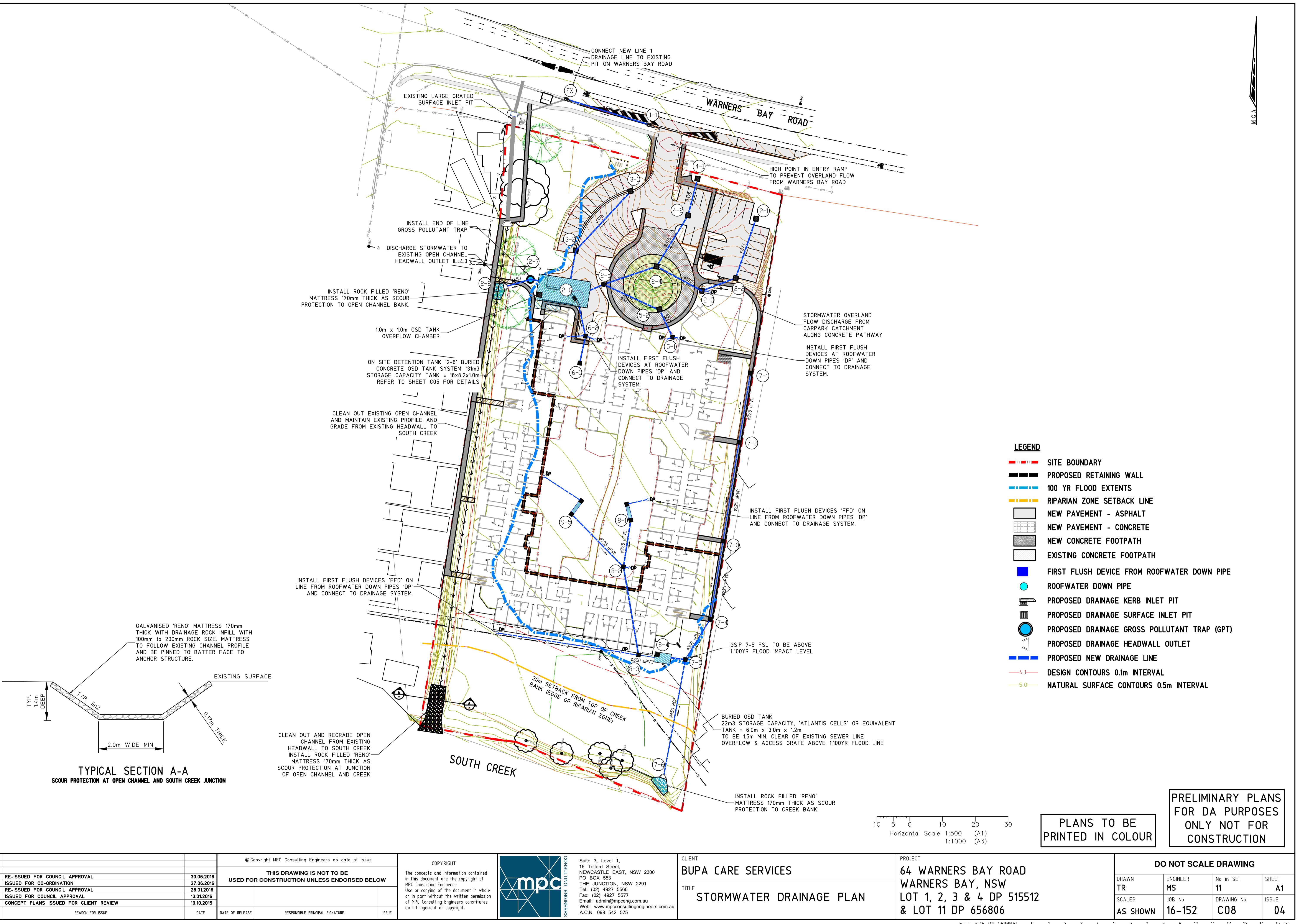


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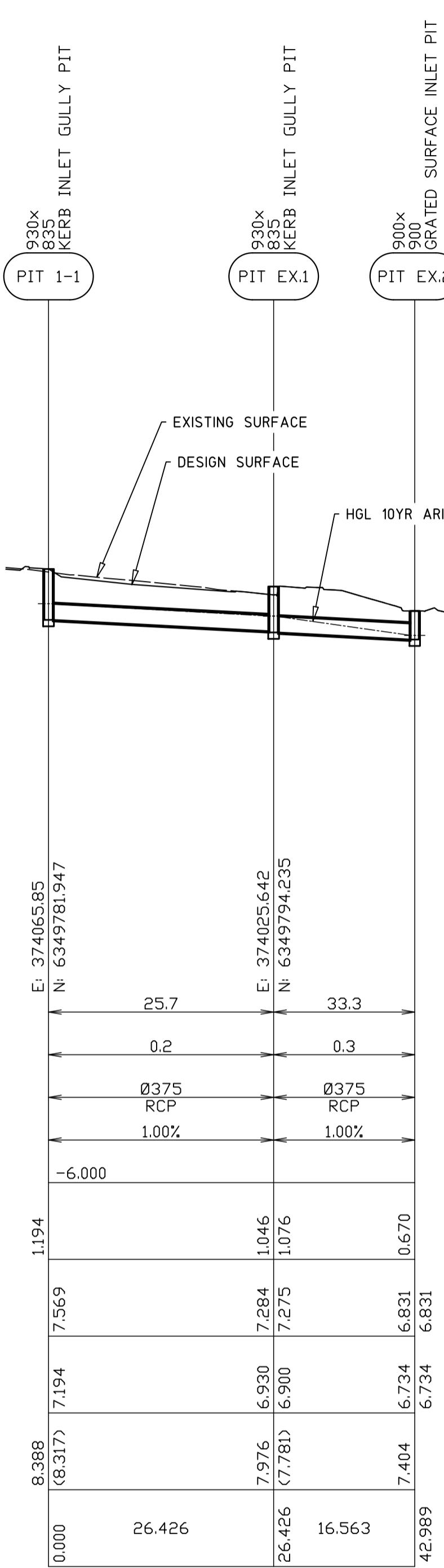
CLIENT  
**BUPA CARE SERVICES**  
 TITLE  
**SITE EARTHWORKS PLAN**

PROJECT  
**64 WARNERS BAY ROAD  
WARNERS BAY, NSW  
LOT 1, 2, 3 & 4 DP 515512  
& LOT 11 DP 656806**

FULL SIZE ON ORIGINAL 0 1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 cm

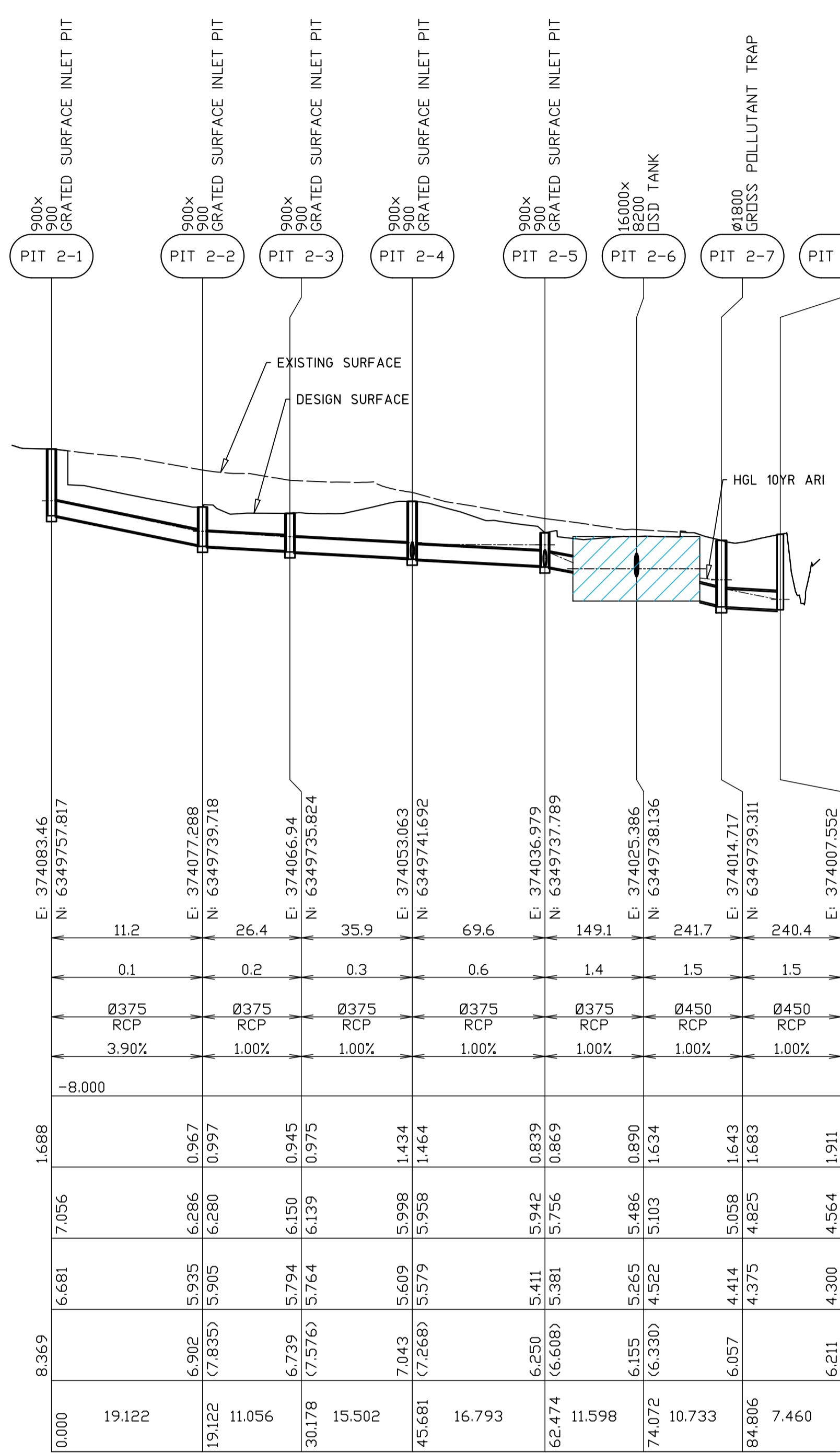


Flow (L/s)  
 Velocity (m/s)  
 Pipe Size/Type  
 Grade (%)  
 Datum  
 Depth To Invert  
 HGL Levels  
 Invert Levels  
 Finished Surface (Natural Surface)  
 Chainage

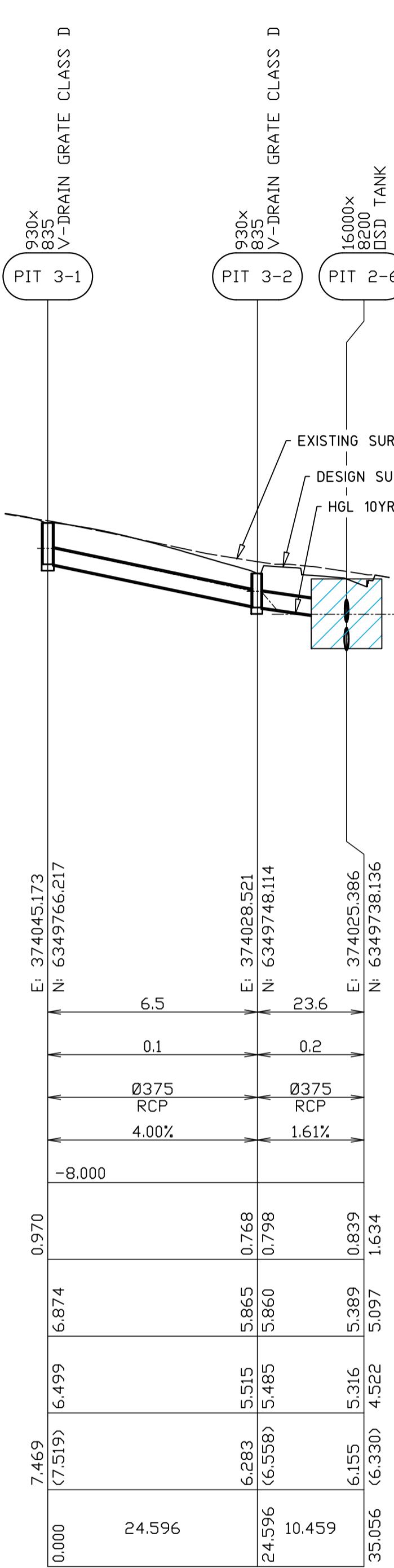


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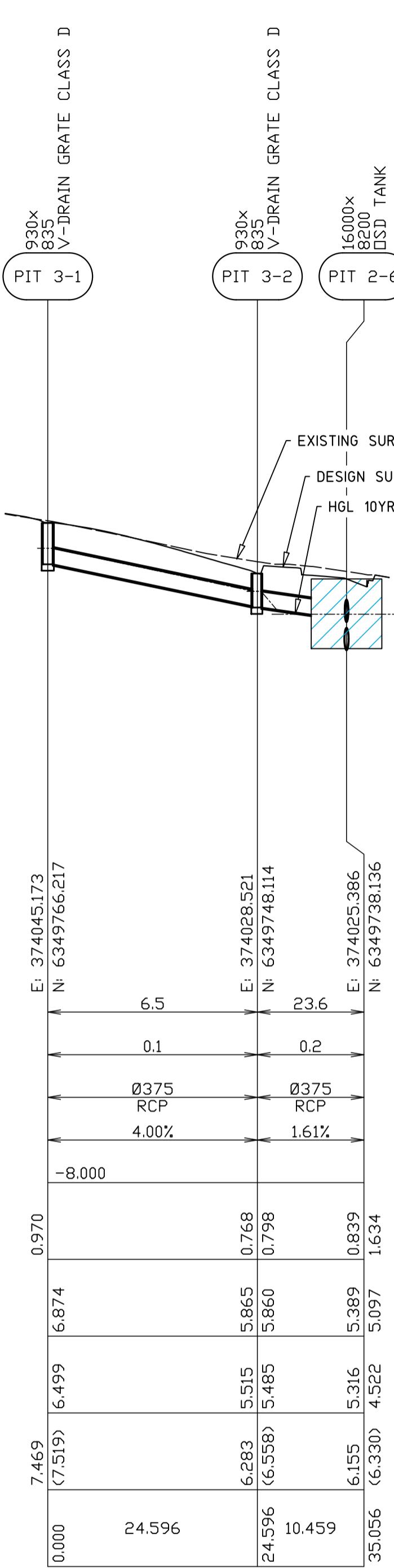
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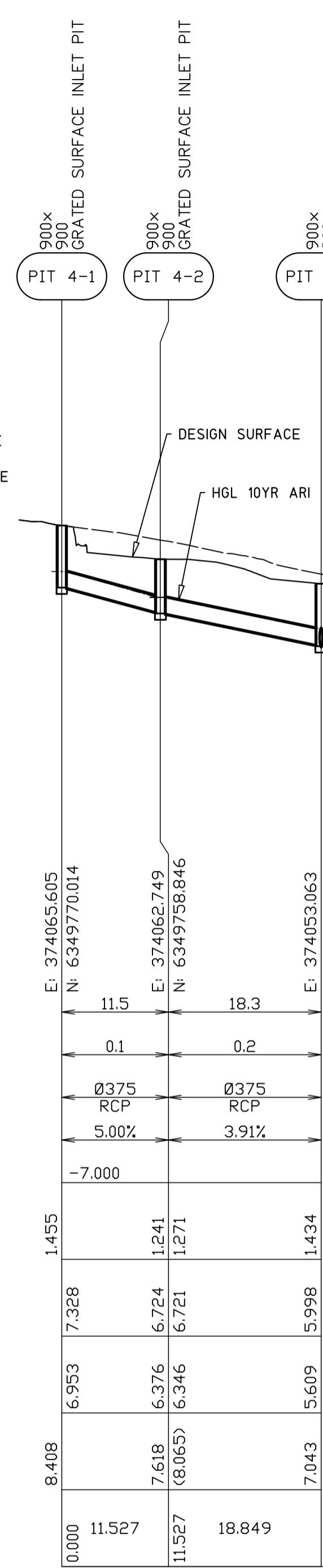
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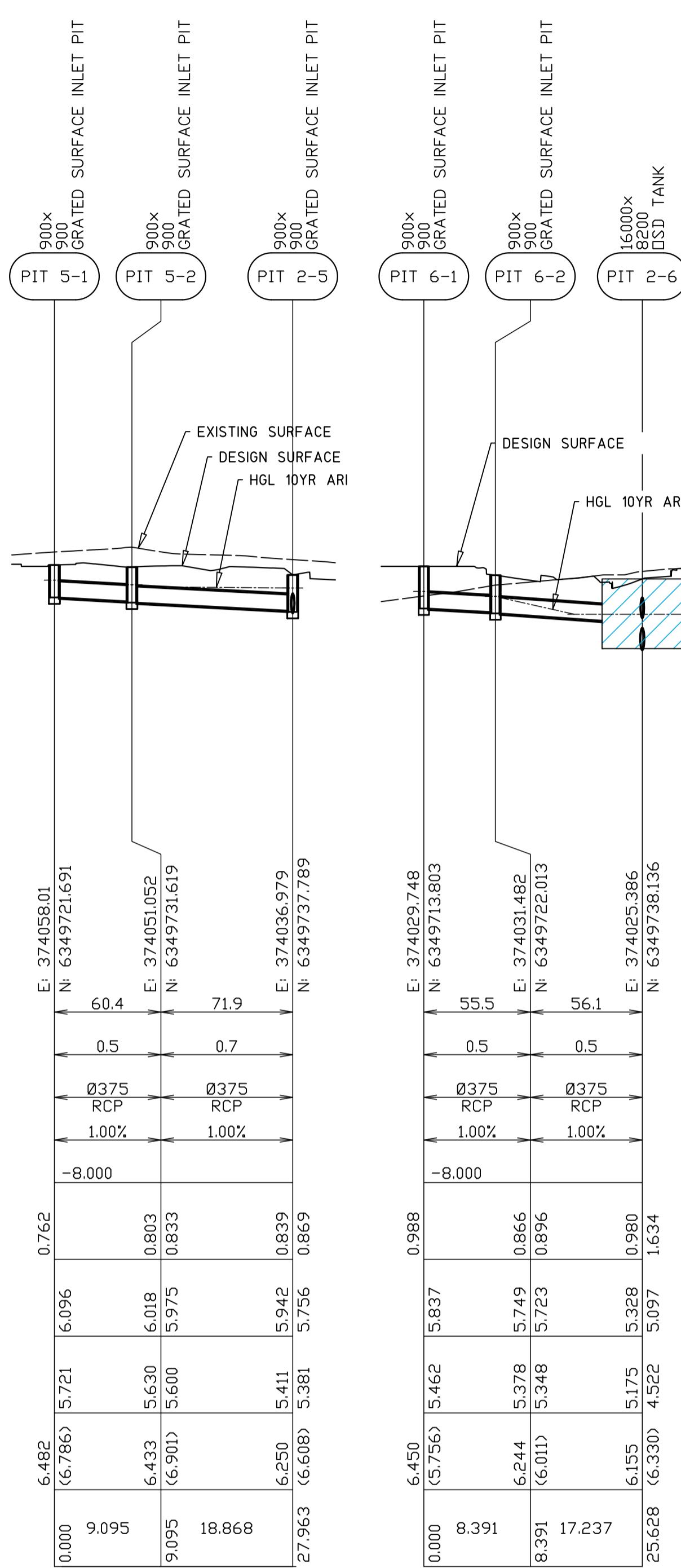
LINE 3



LINE 4



LINE 5



LINE 6

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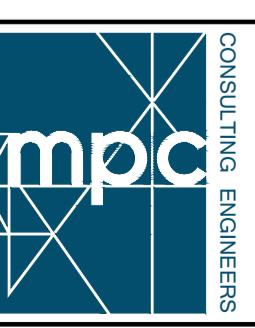
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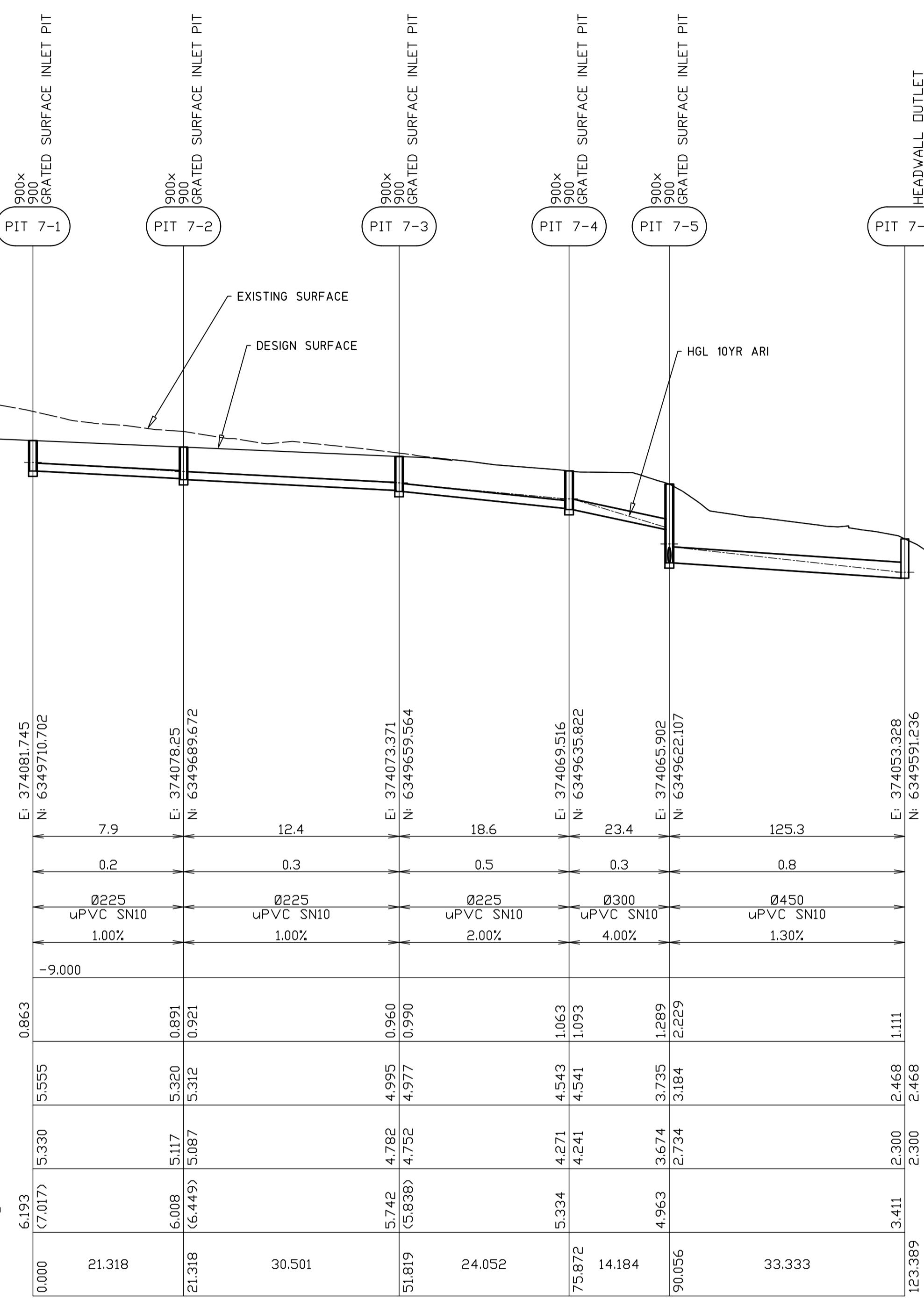
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**PROJECT**  
**64 WARNERS BAY ROAD**  
**WARNERS BAY, NSW**  
**LOT 1, 2, 3 & 4 DP 515512**  
**& LOT 11 DP 656806**  
**TITLE**  
**STORMWATER LONGITUDINAL SECTIONS**  
**SHEET 1 OF 2**

**FULL SIZE ON ORIGINAL**  
 0 1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 cm

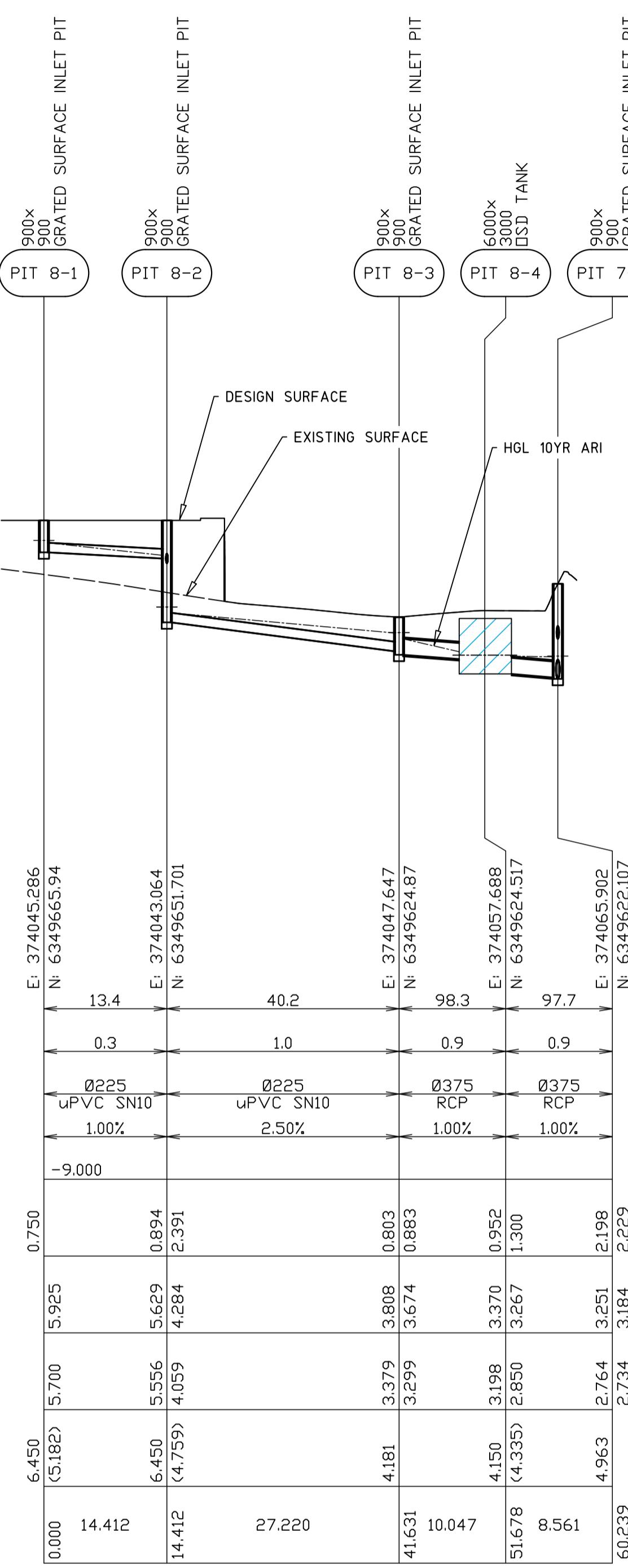
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SCALES AS SHOWN	JOB No 16-152	DRAWING No C09	ISSUE 04

Flow (L/s)  
 Velocity (m/s)  
 Pipe Size/Type  
 Grade (%)  
 Datum  
 Depth To Invert  
 HGL Levels  
 Invert Levels  
 Finished Surface (Natural Surface)  
 Chainage



LINE 7

SCALE: Horizontal 1:500  
Vertical 1:100



LINE 8



LINE 9

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SCALES AS SHOWN	JOB No 16-152	DRAWING No C10	ISSUE 04

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PROJECT  
**64 WARNERS BAY ROAD**  
**WARNERS BAY, NSW**  
**LOT 1, 2, 3 & 4 DP 515512**  
**& LOT 11 DP 656806**  
TITLE  
**STORMWATER LONGITUDINAL SECTIONS**  
**SHEET 2 OF 2**

FULL SIZE ON ORIGINAL 0 1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 cm

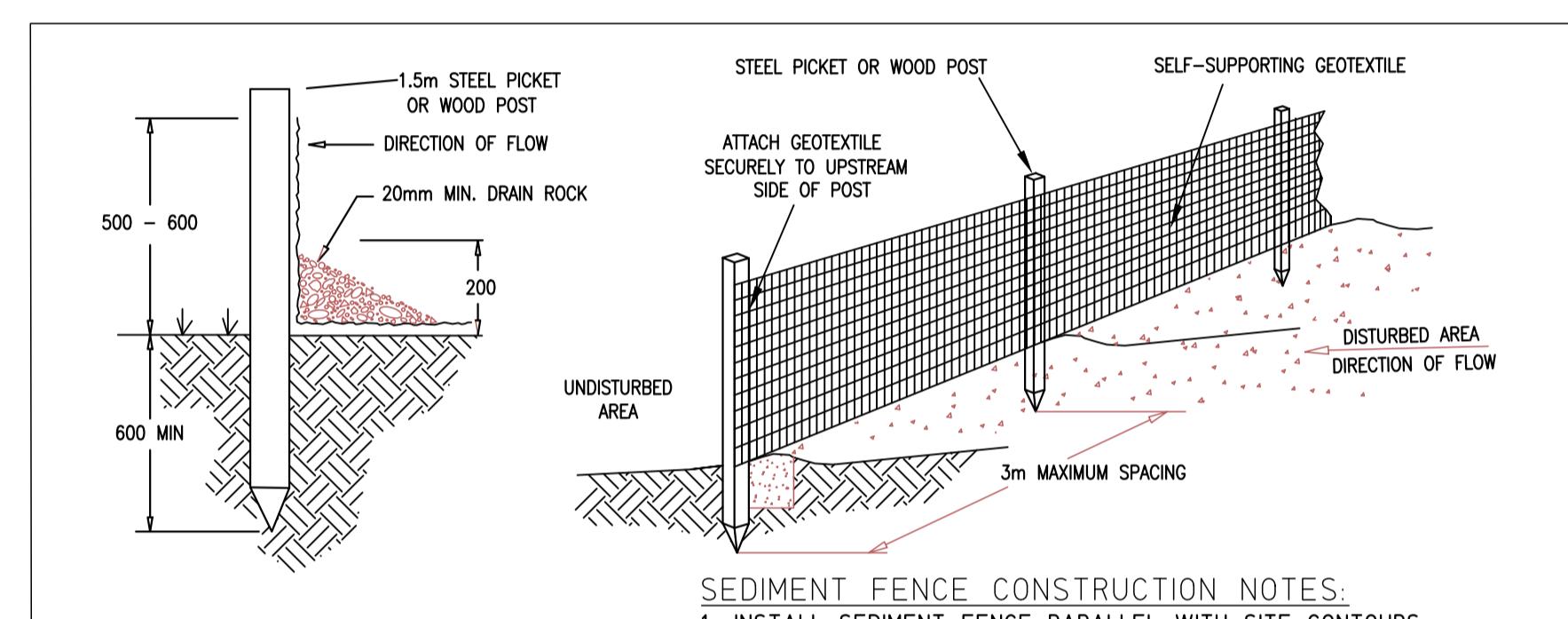


#### GENERAL REHABILITATION NOTES:

- REHABILITATION SHALL BE UNDERTAKEN AS SOON AS POSSIBLE AFTER COMPLETION OF LAND SHAPING AND PREFERABLY WITHIN 7 DAYS.
  - BEFORE PLACING TOPSOIL, THE SURFACE SHALL BE SCARIFIED ALONG THE CONTOUR TO PROVIDE KEYING FOR TOPSOIL.
  - REHABILITATION AREAS WILL BE REGULARLY MAINTAINED UNTIL EFFECTIVE COVER HAS PROPERLY ESTABLISHED. THIS WILL INCLUDE REGULAR WATERING, FERTILISING, WEED CONTROL AND RE-SEEDING/SPRIGGING AS NECESSARY.
  - TEMPORARY SOIL CONSERVATION STRUCTURES SHALL REMAIN & BE REGULARLY MAINTAINED UNTIL ALL WORK HAS BEEN COMPLETED.
  - AS MUCH VEGETATION AS POSSIBLE SHOULD BE MAINTAINED DOWN SLOPE OF "SILT" FENCES AND BELOW DISCHARGE POINTS OF TEMPORARY DIVERSION DRAINS TO ACT AS FILTER STRIPS.
  - DISTURBED AREAS ARE TO BE REVEGETATED WITHIN 14 DAYS OF THE COMPLETION OF EARTHWORKS WITH THE FOLLOWING SEED AND FERTILISER
- LAKE MACQUARIE SPECIAL BLEND  
SEED MIX
- 40% PERENNIAL RYEGRASS  
8% ANNUAL RYEGRASS  
40% UNHULLED COUCH GRASS  
5% CHEWING FESCUE  
7% WOOGANELLUP SUB CLOVER  
RATE: 1kg SEED MIX PER 30m<sup>2</sup>
- HYDRO SEEDING MIX**
- SUMMER  
JAPANESE MILLET (Sept - March) 60kg/ha  
RYECORN (Sept - March) 15kg/ha  
UNHULLED COUCH GRASS 15kg/ha  
HULLED COUCH GRASS 10kg/ha
- WINTER**  
JAPANESE MILLET (April - Oct) 15kg/ha  
RYECORN (April - Oct) 60kg/ha  
UNHULLED COUCH GRASS 15kg/ha  
HULLED COUCH GRASS 10kg/ha
- FERTILISER: DYNAMIC LIFTER OR EQUIV. - 600kg/ha  
FIBRE PULP - 400kg/ha  
FIBRE PULP - 400kg/ha  
NON-REWETTABLE BINDER - TO STOP RAIN WASHING SEED MIX AWAY

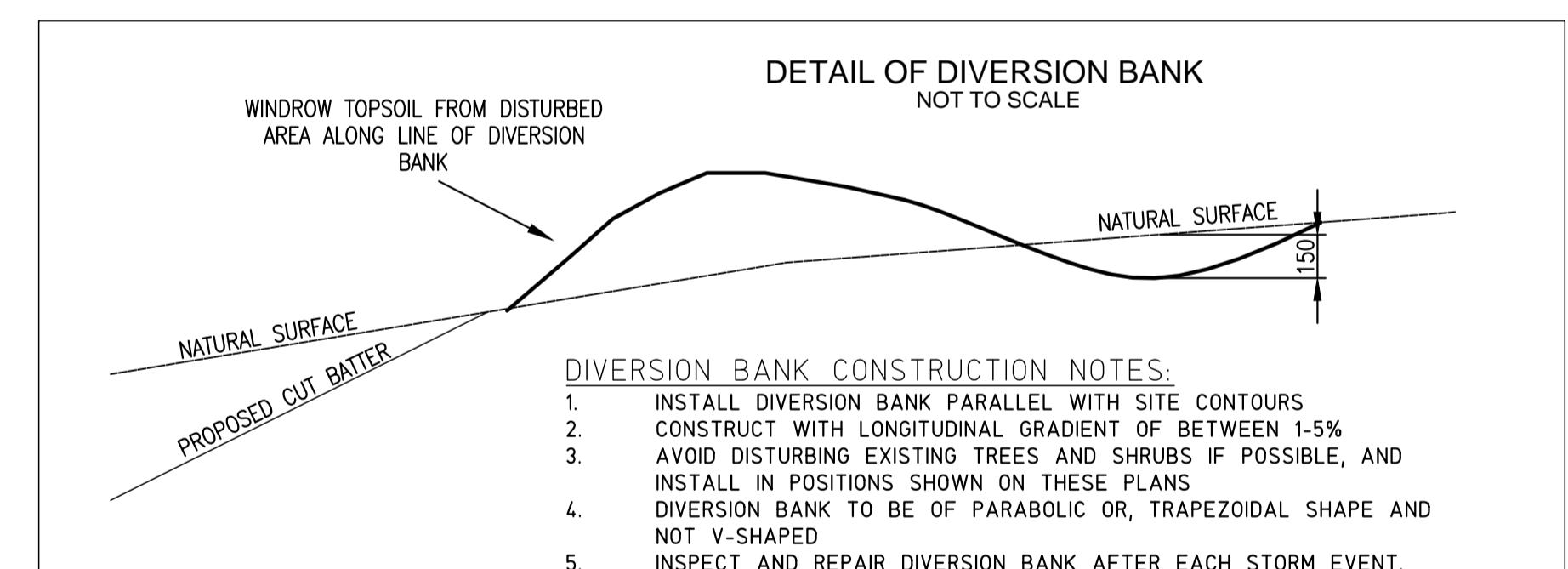
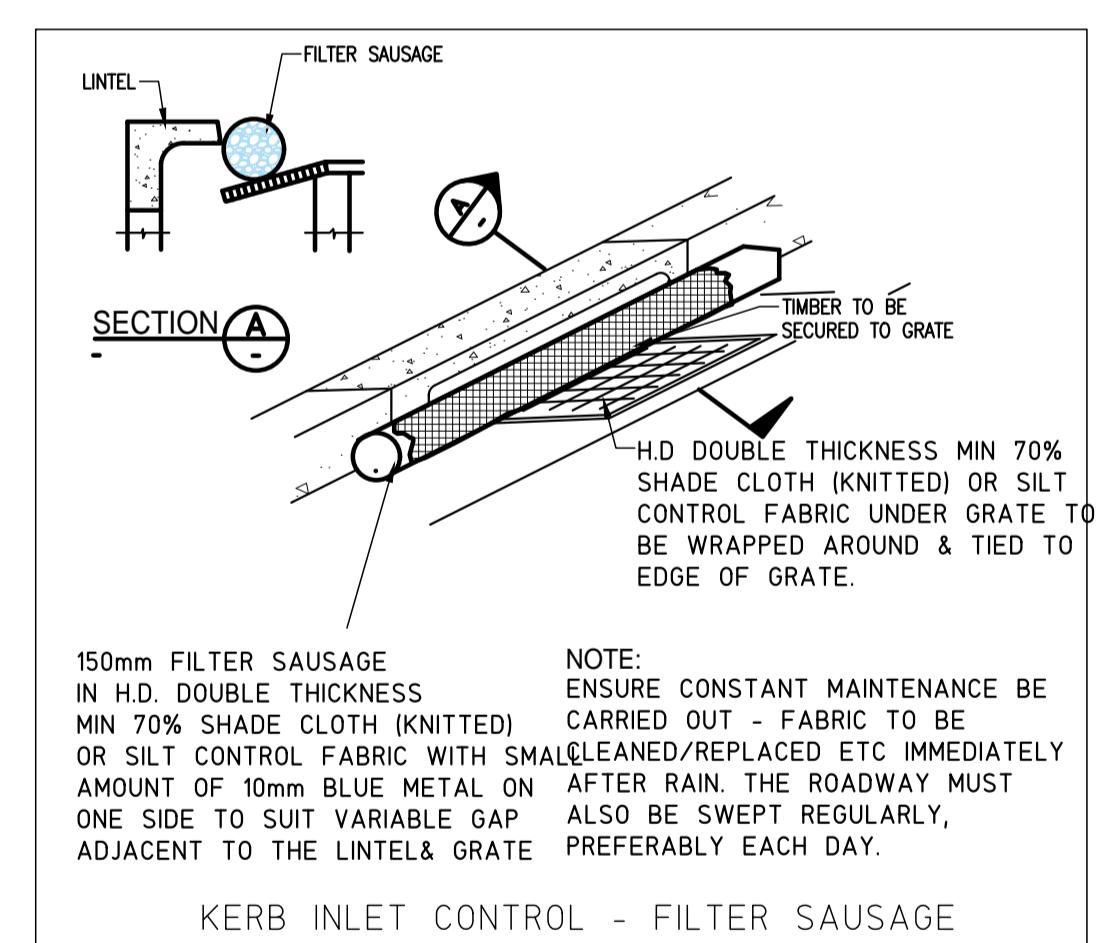
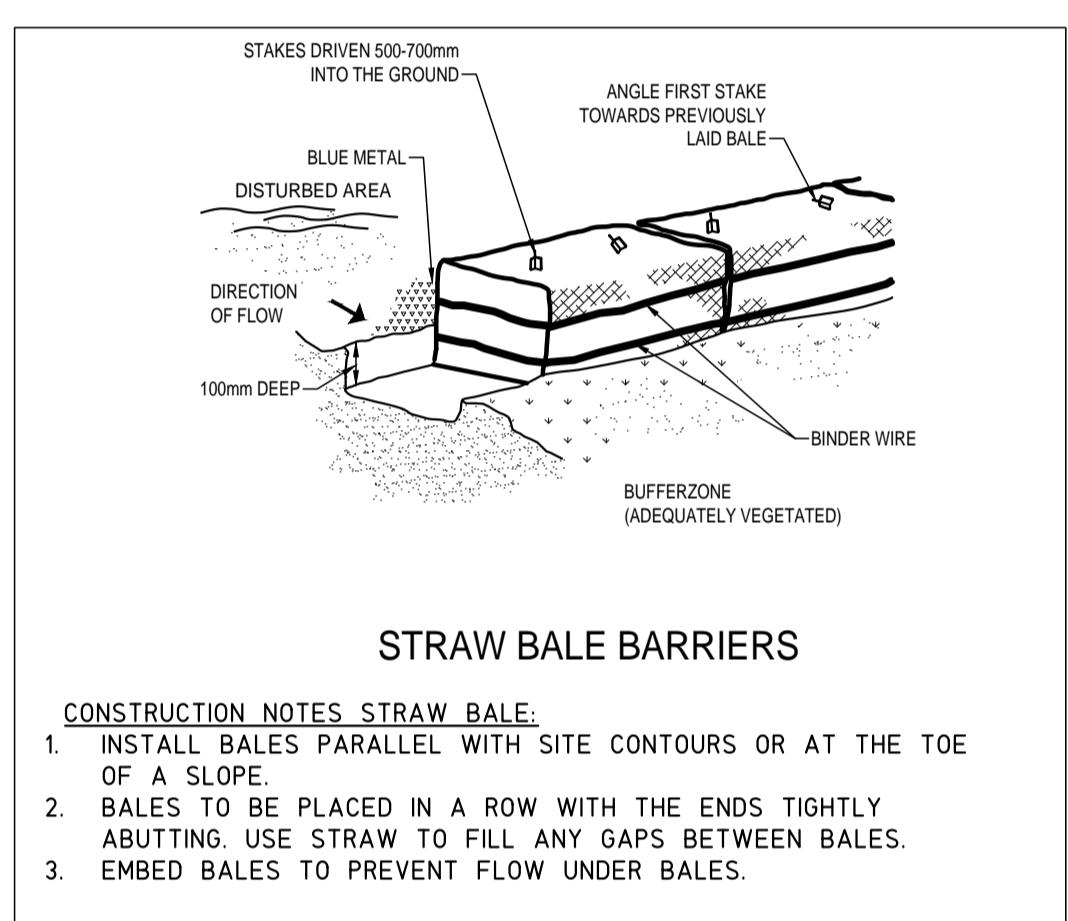
#### EROSION & SEDIMENT CONTROL NOTES:

- SOIL STOCKPILES TO BE NO HIGHER THAN 2.0 METRES (1.0m PREFERABLE) IN LOCATIONS DIRECTED BY THE SUPERINTENDENT.
- CONSTRUCT SEDIMENT FENCE AT LOCATIONS SHOWN AND AS DIRECTED BY SITE SUPERINTENDENT. SEDIMENT FENCE OR EQUIVALENT TO BE CONSTRUCTED AND MAINTAINED IN ACCORDANCE WITH LMCC ENGINEERING REQUIREMENTS.
- UPON COMPLETION OF FINAL EARTHWORKS OR AFTER WRITTEN DIRECTION OF COUNCIL, IMMEDIATE SOIL CONSERVATION TREATMENTS SHALL BE APPLIED SO AS TO RENDER AREAS THAT HAVE BEEN DISTURBED, EROSION PROOF IN 14 DAYS.
- ALL PERIMETER AND SILTATION CONTROL MEASURES ARE TO BE THE FIRST STEP IN CLEARING OR EARTHWORKS.
- SEDIMENTATION BASIN TO BE FLOCULATED AND PUMPED OUT AFTER EVERY STORM EVENT. ALL FLOCULATED SEDIMENT TO BE REMOVED FROM THE BASIN AT THE CONCLUSION OF THE CONTRACT PERIOD.
- ALL TEMPORARY EARTH BERMS AND DIVERSION BANKS ARE TO BE TRACK ROLLED AND SEDED OR MULCHED FOR TEMPORARY VEGETATION COVER AS SOON AS THEY HAVE BEEN FORMED.
- PROVIDE FILTER SAUSAGE KERB INLET SEDIMENT TRAPS OR EQUIVALENT (TO THE SATISFACTION OF THE SUPERINTENDENT) TO ALL CONSTRUCTED INLET PITS AND STORMWATER PIPING.
- ALL TOPSOIL IS TO BE STOCKPILED ON SITE FOR RE-USE (AWAY FROM TREES AND DRAINAGE LINES). MEASURES SHALL BE APPLIED TO PREVENT EROSION FROM THE STOCKPILES.
- SOIL/GRASS - TOPSOIL, 200mm THICK SHALL BE APPLIED TO ALL DISTURBED AREAS. ALL REMAINING EXPOSED TOPSOIL SHALL BE SEDED IMMEDIATELY UPON COMPLETION OF THE SOIL SPREADING OPERATION.



**SEDIMENT FENCE CONSTRUCTION NOTES:**

- INSTALL SEDIMENT FENCE PARALLEL WITH SITE CONTOURS.
- JOIN SECTIONS OF FABRIC AT A SUPPORT POST WITH 150mm OVERLAP.
- PLACE ON CONTOUR WITH SLIGHT CONVEX TO THE CONTOUR TO FORM A POND FOR WATER COLLECTION. EACH FENCE TO BE NO LONGER THAN 20m, SHOULD BE OVERLAPPING AND SHOULD NOT CAPTURE LARGE OR CONCENTRATED FLOWS.
- INSPECT AND REPAIR FENCE AFTER EACH STORM EVENT AND REMOVE SEDIMENT WHEN NECESSARY.



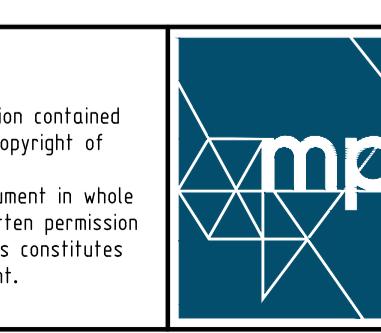
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CLIENT  
**BUPA CARE SERVICES**  
TITLE  
**EROSION & SEDIMENTATION  
CONTROL PLAN**

PROJECT  
**64 WARNERS BAY ROAD  
WARNERS BAY, NSW  
LOT 1, 2, 3 & 4 DP 515512  
& LOT 11 DP 656806**

Horizontal Scale 1:500 (A1)  
1:1000 (A3)

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DRAWN TR	ENGINEER MS	No in SET 11	Sheet A1
SCALES AS SHOWN	JOB No 16-152	DRAWING No C11	ISSUE 04

FULL SIZE ON ORIGINAL 0 1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 cm

## **Appendix A**

### **Water Quality Treatment Train**

